

## SAB2223 Mechanics of Materials and Structures

## TOPIC 2 SHEAR FORCE AND BENDING MOMENT

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# TOPIC 2 SHEAR FORCE AND BENDING MOMENT



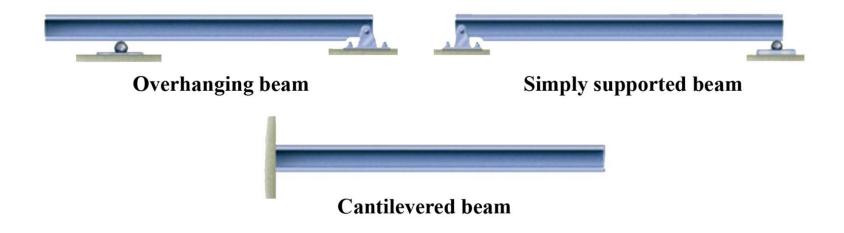


- Introduction
  - Types of beams
  - Effects of loading on beams
  - The force that cause shearing is known as shear force
  - The force that results in bending is known as bending moment
  - Draw the shear force and bending moment diagrams





- Members with support loadings applied perpendicular to their longitudinal axis are called beams.
- Beams classified according to the way they are supported.





#### Types of beam

#### a) Determinate Beam

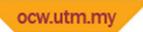
The force and moment of reactions at supports can be determined by using the 3 equilibrium equations of statics i.e

$$\Sigma F_x = 0$$
 ,  $\Sigma F_y = 0$  , and  $\Sigma M = 0$ 

#### b) Indeterminate Beam

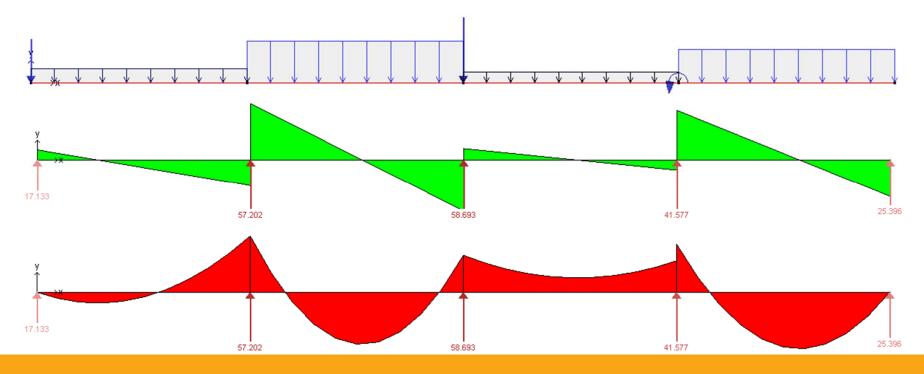
The force and moment of reactions at supports are more than the number of equilibrium equations of statics.

(The extra reactions are called redundant and represent the amount of degrees of indeterminacy).





• In order to properly design a beam, it is important to know the *variation* of the shear and moment along its axis in order to find the points where these values are a maximum.







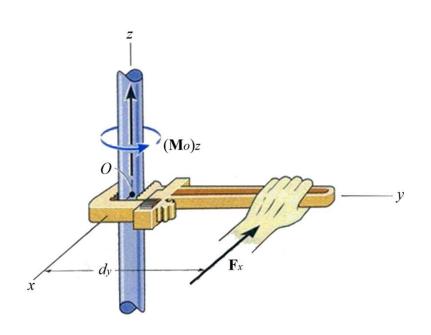
#### **Principle of Moments**

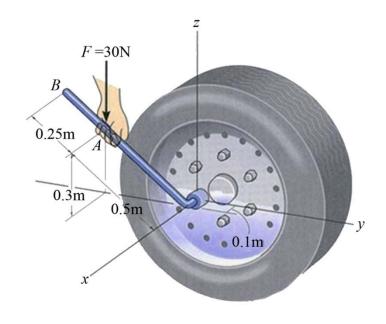
- The moment of a force indicates the tendency of a body to turn about an axis passing through a specific point O.
- The principle of moments, which is sometimes referred to as *Varignon's Theorem* (Varignon, 1654 1722) states that *the moment of a force about a point is equal to the sum of the moments of the force's components about the point*.





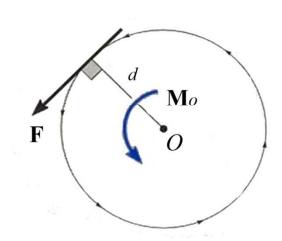
## **Principle of Moments**





In the 2-D case, the magnitude of the moment is

$$M_o = F d$$



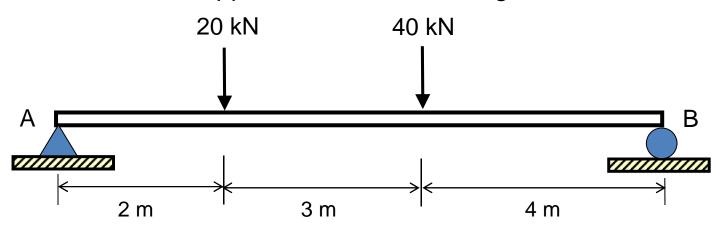


#### **Beam's Reactions**

- If a support *prevents translation* of a body in a particular direction, then the support exerts a *force* on the body in that direction.
- Determined using  $\Sigma F_x$ =0 ,  $\Sigma F_y$ =0 , and  $\Sigma M$ =0

#### Example 1:

The beam shown below is supported by a pin at A and roller at B. Calculate the reactions at both supports due to the loading.

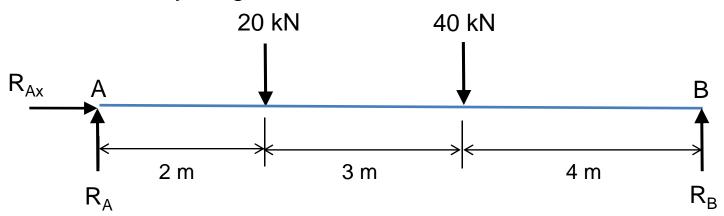




#### **Example 1**

#### **Solution**

Draw free body diagram



By taking the moment at B,

$$\Sigma M_{\rm B} = 0$$

$$R_A \times 9 - 20 \times 7 - 40 \times 4 = 0$$
  $R_A + R_B - 20 - 40 = 0$ 

$$9R_{\rm A} = 140 + 160$$

$$R_{\rm A} = 33.3 \; {\rm kN}$$

$$\Sigma F_{\rm v} = 0$$

$$R_{\rm A} + R_{\rm B} - 20 - 40 = 0$$

$$R_{\rm B} = 20 + 40 - 33.3$$

$$R_{\rm B} = 26.7 \; {\rm kN}$$

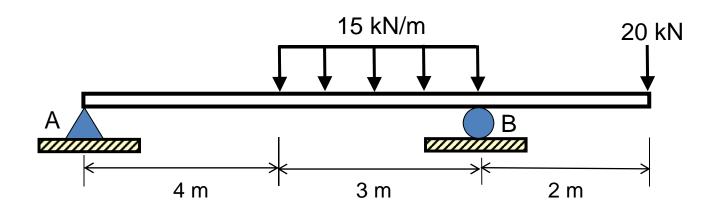
$$\Sigma F_{\rm x} = 0$$

$$R_{\rm Ax} = 0$$





Determine the reactions at support A and B for the overhanging beam subjected to the loading as shown.

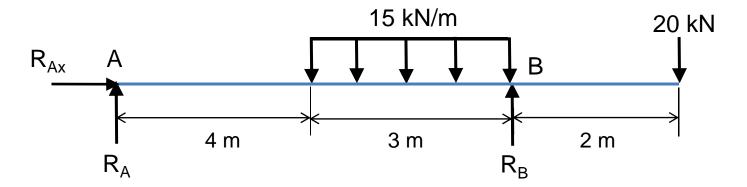




#### Example 2 (cont.)

#### Solution

Draw free body diagram



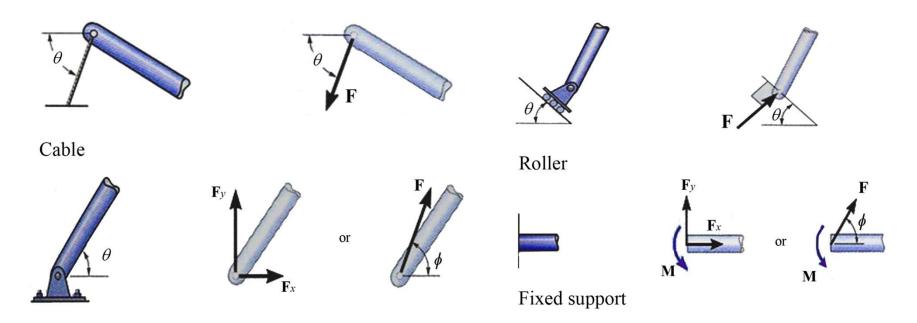
By taking the moment at A,

$$\Sigma M_{\rm A} = 0$$
  $\Sigma F_{\rm y} = 0$   $\Sigma F_{\rm x} = 0$   $-R_{\rm B} \times 7 + 20 \times 9 - (15 \times 3) \times 5.5 = 0$   $R_{\rm A} + R_{\rm B} - 20 - 45 = 0$   $R_{\rm Ax} = 0$   $R_{\rm A} = 20 + 45 - 61.07$   $R_{\rm A} = 3.93 \; {\rm kN}$ 





## **Types of Support**

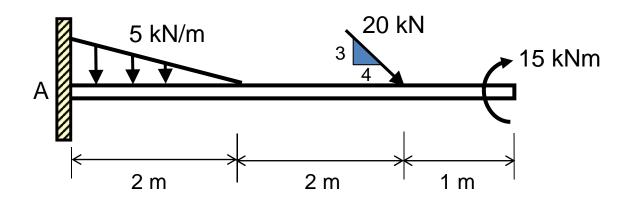


As a general rule, if a *support prevents translation* of a body in a given direction, then *a force is developed* on the body in the opposite direction. Similarly, if *rotation is prevented, a couple moment* is exerted on the body.





A cantilever beam is loaded as shown. Determine all reactions at support A.

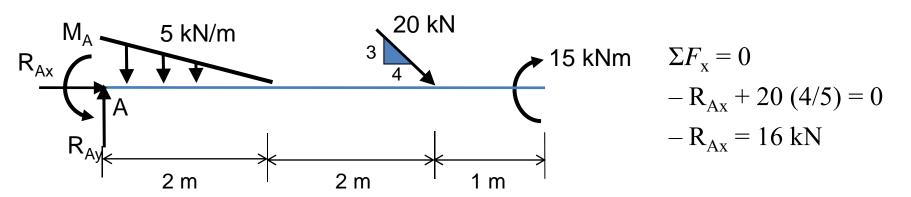




#### Example 3 (cont.)

#### Solution

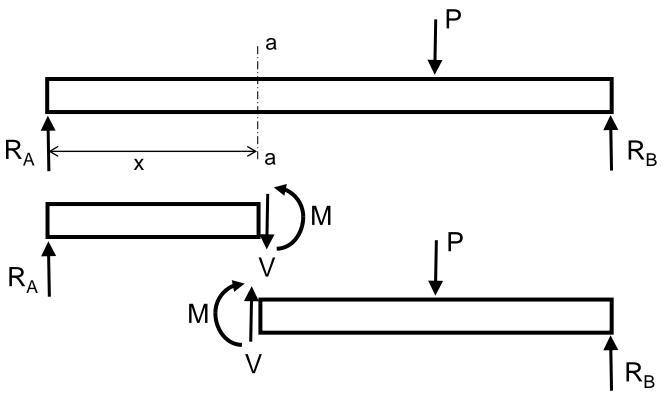
Draw free body diagram



$$\Sigma F_{y} = 0$$
  $\Sigma M_{A} = 0$   $R_{Ay} - 0.5 (5)(2) - 20(3/5) = 0$   $-M_{A} + 0.5(5)(2)(1/3)(2) + 20(3/5) (4) + 15 = 0$   $R_{Ay} - 5 - 12 = 0$   $M_{A} = 3.3 + 48 + 15$   $R_{Ay} = 17 \text{ kN}$   $M_{A} = 66.3 \text{ kNm}$ 







V = shear force

= the force that tends to separate the member

= balances the reaction  $R_A$ 



- M = bending moment
  - = the reaction moment at a particular point (section)
  - = balances the moment,  $R_{\Delta}$ .x

From the equilibrium equations of statics,

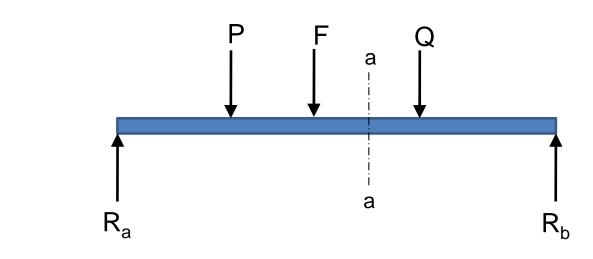
$$+\int \Sigma F_y = 0;$$
  $R_A - V$ 

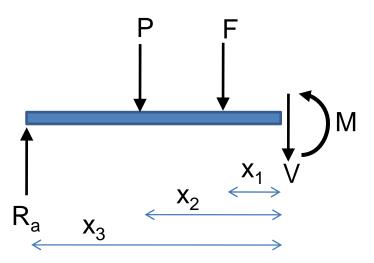
$$\therefore V = R_{A}$$

$$+ \hat{\Sigma} F_y = 0;$$
  $R_A - V = 0$   $\therefore V = R_A$   
 $+ \hat{\Sigma} M_a = 0;$   $-M + R_A \cdot x = 0$   $\therefore M = R_A \cdot x$ 

$$\therefore M = R_A.x$$







$$\Sigma F_{y} = 0$$

$$R_{a} - P - F - V = 0$$

$$V = R_{a} - P - F$$

$$\Sigma M_a = 0 - M - F.x_1 - P.x_2 + R_a.x_3 = 0 M = R_a.x_3 - F.x_1 - P.x_2$$



Shape deformation due to shear force





Shape deformation due to bending moment





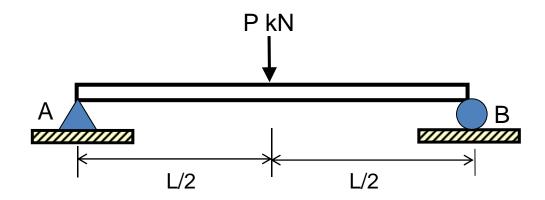
- Positive shear force diagram drawn above the beam
- Positive bending moment diagram drawn below the beam





#### **Example 4**

- a) Calculate the shear force and bending moment for the beam subjected to a concentrated load as shown in the figure, and draw the shear force diagram (SFD) and bending moment diagram (BMD).
- b) If P = 20 kN and L = 6 m, draw the SFD and BMD for the beam.



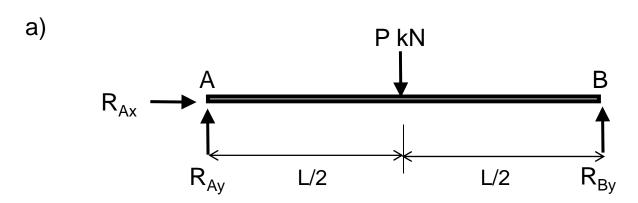


#### Example 4 (cont.)

 $\Sigma F_{\rm x} = 0$ 

 $R_{\rm Ax} = 0$ 

#### **Solution**



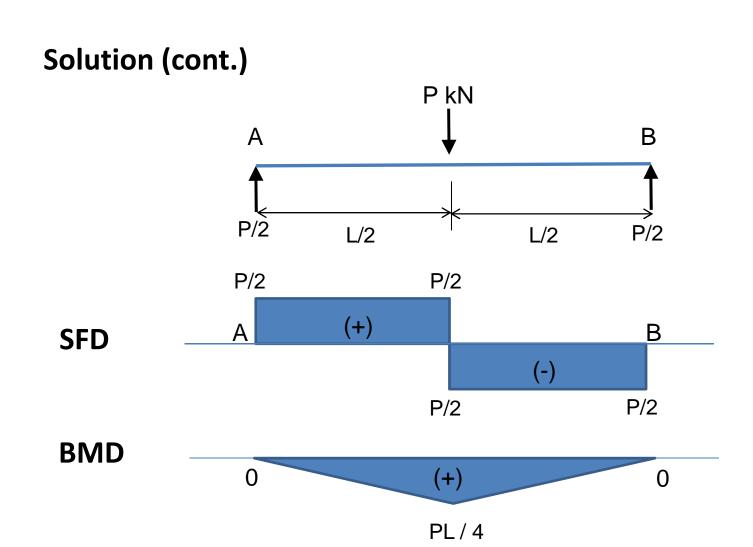
By taking the moment at A,

$$\Sigma M_{\rm A} = 0$$
  $\Sigma F_{\rm y} = 0$   $-R_{\rm B} \times L + P \times L/2 = 0$   $R_{\rm A} + R_{\rm B} = P$   $R_{\rm B} = P/2 \text{ kN}$   $R_{\rm A} = P - P/2$   $R_{\rm A} = P/2 \text{ kN}$ 





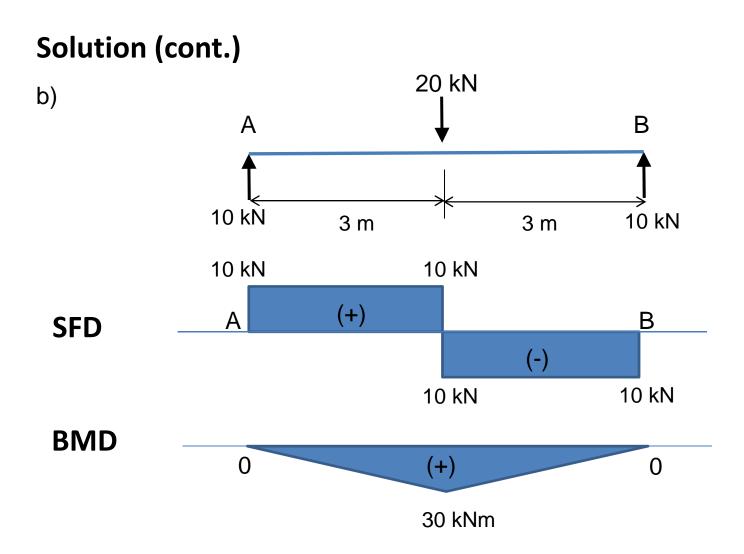
## Example 4 (cont.)







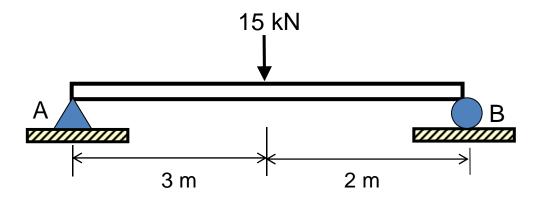
## Example 4 (cont.)







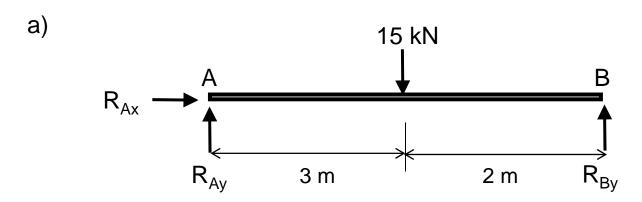
Calculate the shear force and bending moment for the beam subjected to a concentrated load as shown in the figure, then draw the shear force diagram (SFD) and bending moment diagram (BMD).





## Example 5 (cont.)

#### **Solution**



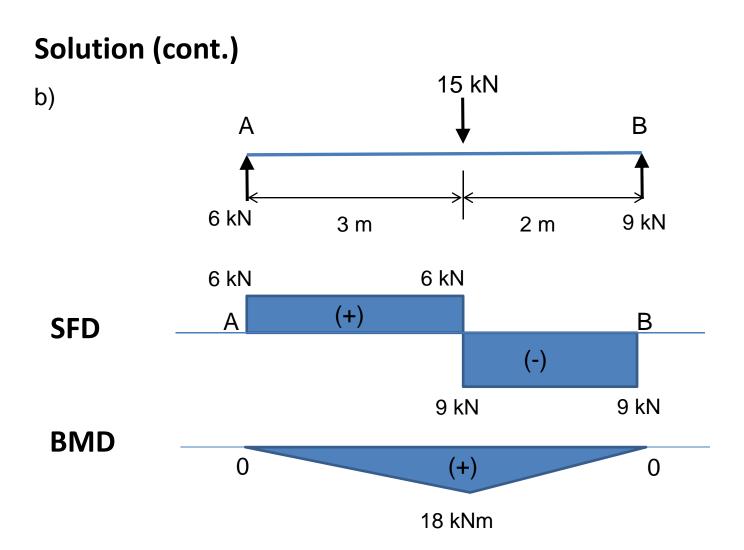
By taking the moment at A,

$$\Sigma M_{\rm A} = 0$$
  $\Sigma F_{\rm y} = 0$   $\Sigma F_{\rm x} = 0$   $-R_{\rm B} \times 5 + 15 \times 3 = 0$   $R_{\rm A} + R_{\rm B} = 15$   $R_{\rm Ax} = 0$   $R_{\rm A} = 15 - 9$   $R_{\rm A} = 6 \text{ kN}$ 





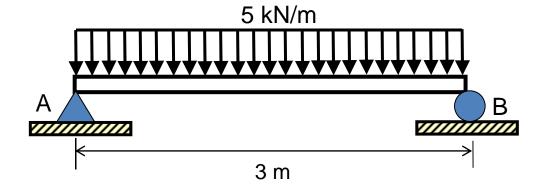
## Example 5 (cont.)







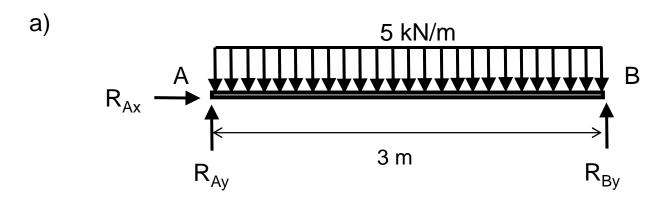
Calculate the shear force and bending moment for the beam subjected to an uniformly distributed load as shown in the figure, then draw the shear force diagram (SFD) and bending moment diagram (BMD).





## Example 6 (cont.)

#### **Solution**



By taking the moment at A,

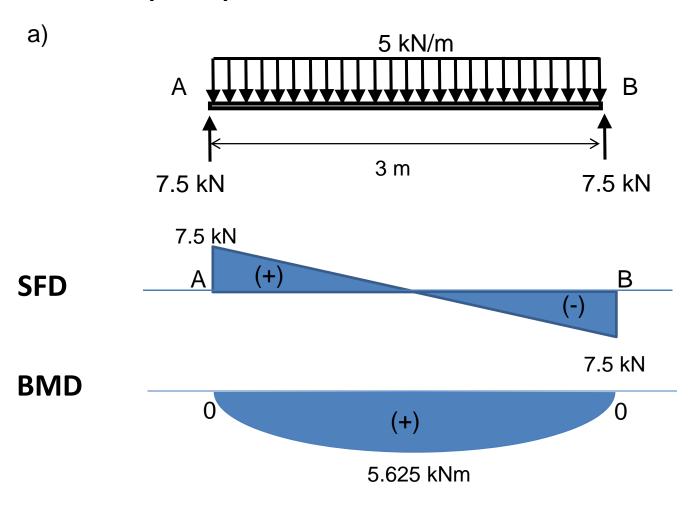
$$\Sigma M_{\rm A} = 0$$
  $\Sigma F_{\rm y} = 0$   $\Sigma F_{\rm x} = 0$   $-R_{\rm B} \times 3 + 5 \times 3 \times 3/2 = 0$   $R_{\rm A} + R_{\rm B} = 5 \times 3$   $R_{\rm Ax} = 0$   $R_{\rm A} = 7.5 \text{ kN}$   $R_{\rm A} = 7.5 \text{ kN}$ 





## Example 6 (cont.)

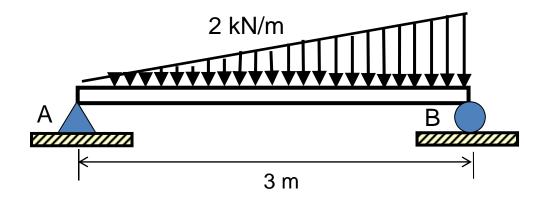
#### Solution (cont.)







Calculate the shear force and bending moment for the beam subjected to the loads as shown in the figure, then draw the shear force diagram (SFD) and bending moment diagram (BMD).

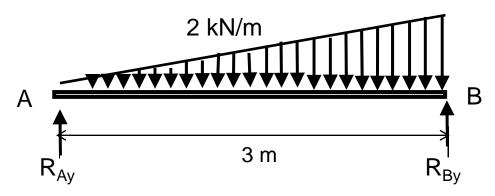






## Example 7 (cont.)

#### **Solution**



By taking the moment at A,

$$\Sigma M_{A} = 0$$

$$2 \times 3/2 \times 3 \times 2/3 - R_{B} \times 3 = 0$$

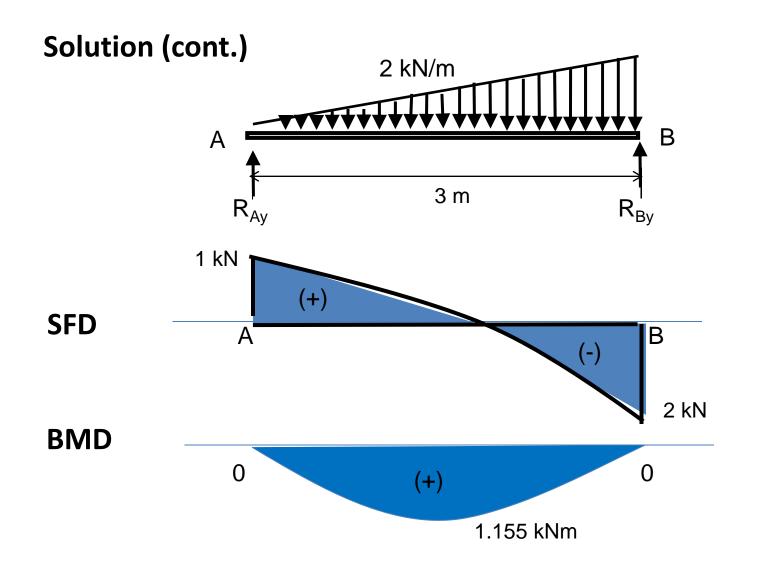
$$R_{B} = 2 \text{ kN}$$

$$\Sigma F_{y} = 0$$
  $\Sigma F_{x} = 0$   $R_{A} + R_{B} = 2 \times 3/2$   $R_{Ax} = 0$   $R_{A} = 3 - 2$   $R_{A} = 1 \text{ kN}$ 





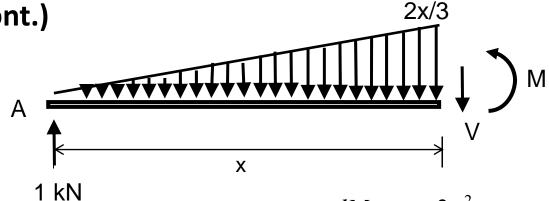
## Example 7 (cont.)





## Example 7 (cont.)





$$1 - 2x/3(x)(1/2) - V = 0$$

$$V = 1 - 2x^2/6$$

If x = 0, V = 1 kN and x = 3, V = -2 kN

$$- M + 1 \times x - 2x/3(x)(1/2)(x/3) = 0$$

$$M = x - x^3/9$$

$$M = \text{maximum when } \frac{dM}{dx} = 0$$

$$\frac{dM}{dx} = 1 - \frac{3x^2}{9} = 0$$
$$x^2 = \frac{9}{3}$$

$$x = \frac{3}{\sqrt{3}} = 1.732m$$

Therefore, M maximum

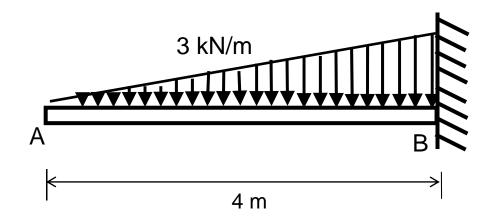
$$M = (1.732) - (1.732)^3/9$$

$$M = 1.155 \text{ kNm}$$





Calculate the shear force and bending moment for the beam subjected to the loads as shown in the figure, then draw the shear force diagram (SFD) and bending moment diagram (BMD).

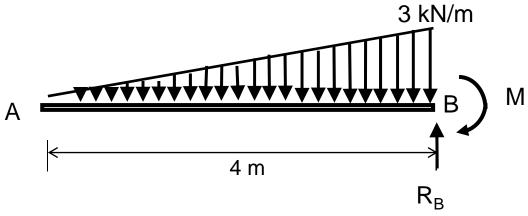






## Example 8 (cont.)

#### **Solution**



By taking the moment at B,

$$\Sigma M_{\rm B} = 0$$

$$M_{\rm B} = 3 \times 4/2 \times 4/3$$

$$M_{\rm B} = 8 \text{ kNm}$$

$$\Sigma F_{y} = 0$$

$$R_{B} = 3 \times 4/2$$

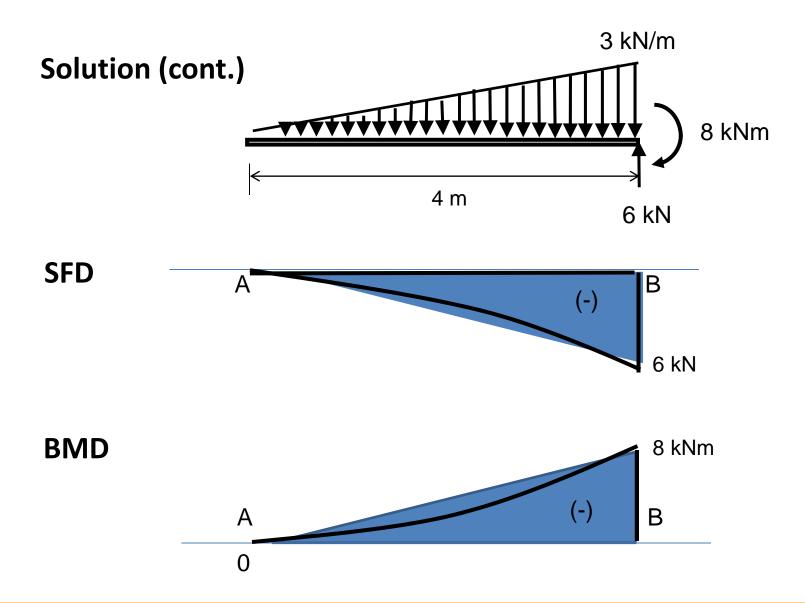
$$R_{B} = 6 \text{ kN}$$

$$\Sigma F_{x} = 0$$
$$R_{Bx} = 0$$



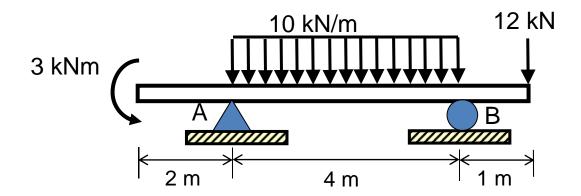


## Example 8 (cont.)







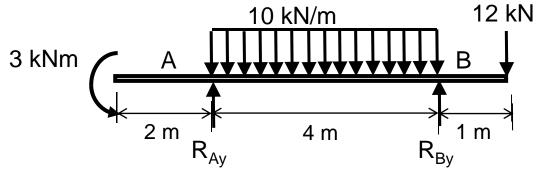






# Example 9 (cont.)

#### **Solution**



$$\Sigma M_{\rm A} = 0$$

$$-R_{\rm B} \times 4 - 3 + 10 \times 4 \times 4/2 + 12 \times 5 = 0$$

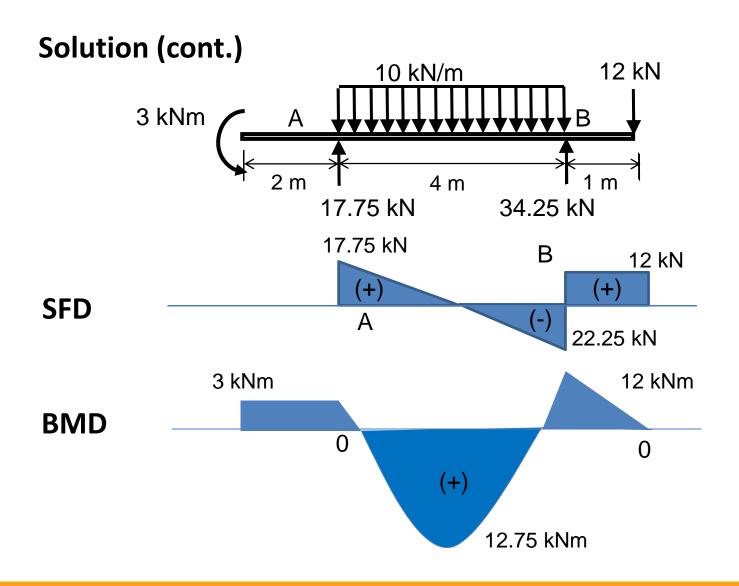
$$R_{\rm B} = 34.25 \text{ kN}$$

$$\Sigma F_{y} = 0$$
  $\Sigma F_{x} = 0$   $R_{A} + R_{B} = 10 \times 4 + 12$   $R_{Ax} = 0$   $R_{A} = 52 - 34.25$   $R_{A} = 17.75 \text{ kN}$ 





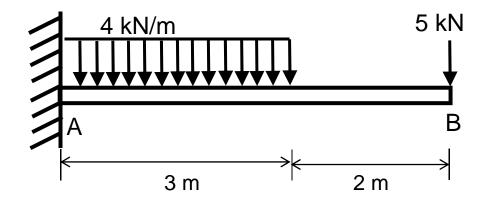
# Example 9 (cont.)







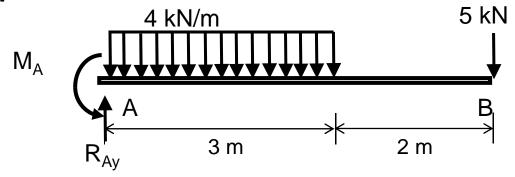
# **Example 10**





# Example 10 (cont.)

#### **Solution**



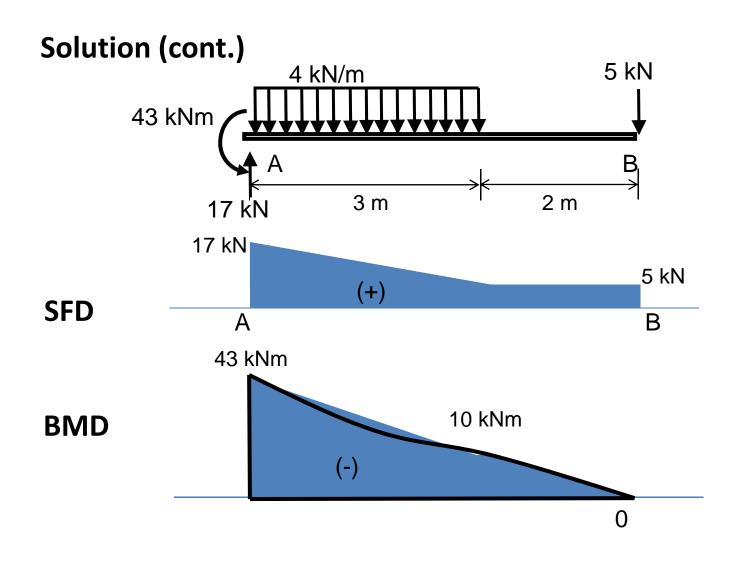
$$\Sigma M_{\rm A} = 0$$
  
 $-M_{\rm A} + 4 \times 3 \times 3/2 + 5 \times 5 = 0$   
 $M_{\rm A} = 43 \text{ kNm}$ 

$$\Sigma F_{y} = 0$$
  $\Sigma F_{x} = 0$   $R_{A} = 4 \times 3 + 5$   $R_{Ax} = 0$   $R_{Ax} = 12 + 5$   $R_{A} = 17 \text{ kN}$ 



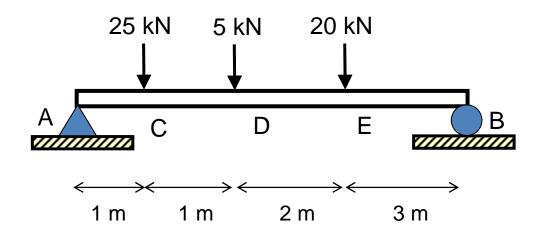


# Example 10 (cont.)



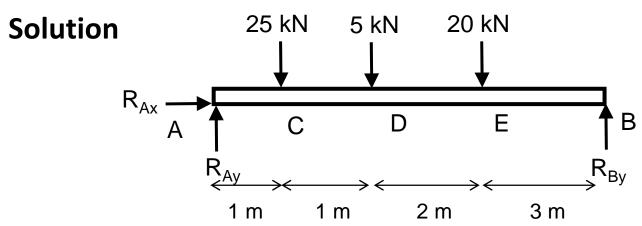








## Example 11 (cont.)



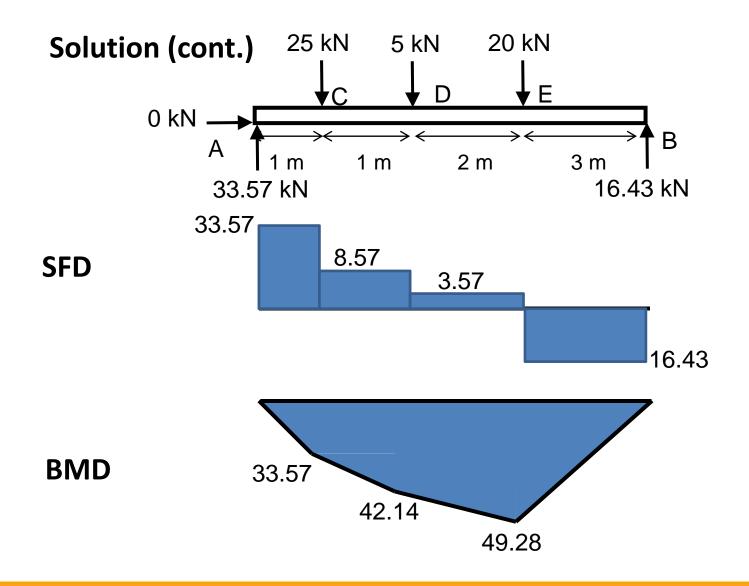
$$\Sigma M_{A} = 0$$
  $\Sigma R_{By} = 16.43 \text{ kN}$   $\Sigma R_{By} = 16.43 \text{ kN}$ 

$$\Sigma F_{y} = 0$$
  $\Sigma F_{x} = 0$   $R_{Ay} + R_{By} = 25 + 5 + 20$   $R_{Ax} = 0$   $R_{Ay} = 50 - 16.43$   $R_{Ay} = 33.57 \text{ kN}$ 



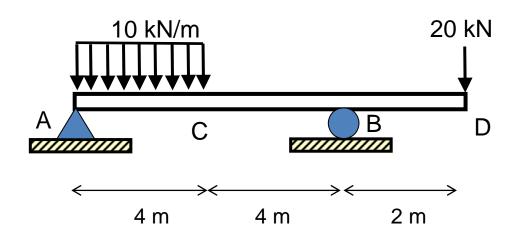


# Example 11 (cont.)



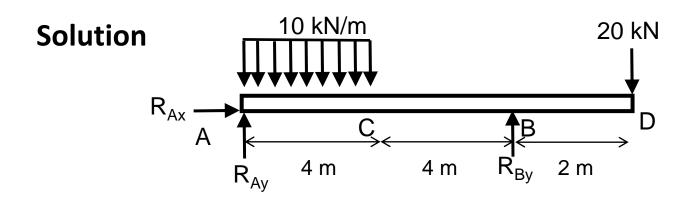








# Example 12 (cont.)



$$\Sigma M_{\rm A} = 0$$

$$10 \times 4 \times 2 + 20 \times 10 - R_{\rm By} \times 8 = 0$$

$$R_{\rm By} = 35 \text{ kN}$$

$$\Sigma F_{y} = 0$$

$$R_{Ay} + R_{By} = 10 \times 4 + 20$$

$$R_{Ax} = 0$$

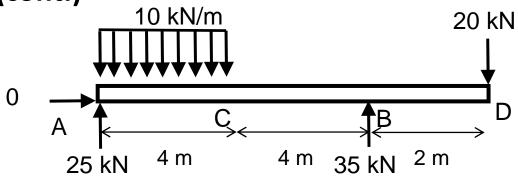
$$R_{Ay} = 60 - 35$$

$$R_{Ay} = 25 \text{ kN}$$



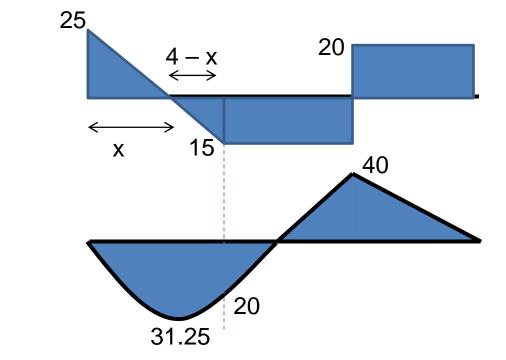
# Example 12 (cont.)





**SFD** 

**BMD** 



$$\frac{x}{25} = \frac{4 - x}{15}$$

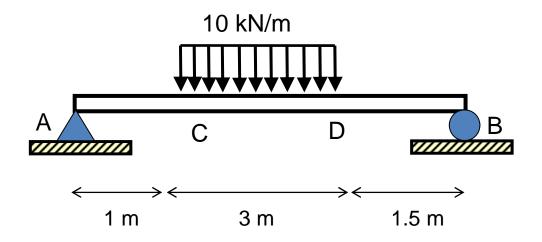
$$15x = 100 - 25x$$

$$40x = 100$$

$$x = 2.5$$



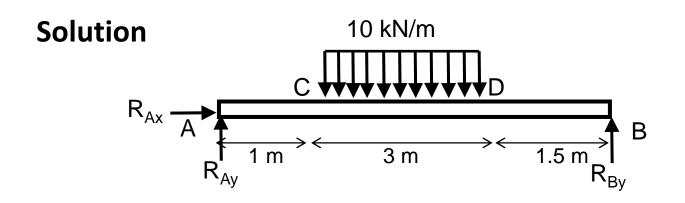








# Example 13 (cont.)

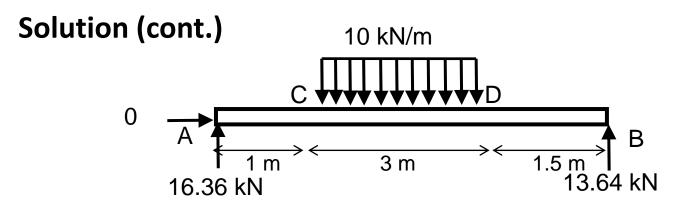


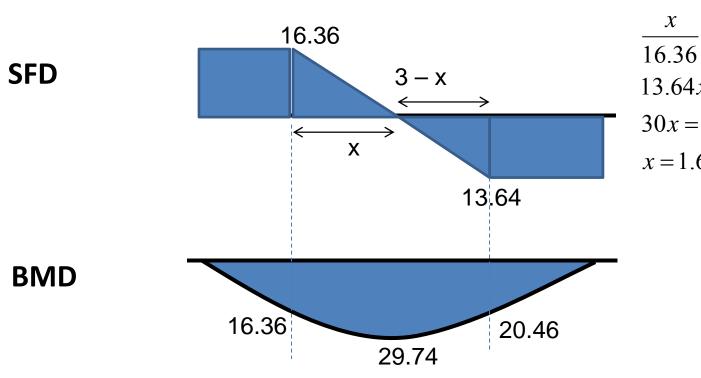
$$\Sigma M_{\rm A} = 0$$
  
 $10 \times 3 \times 2.5 - R_{\rm By} \times 5.5 = 0$   
 $R_{\rm By} = 13.64 \text{ kN}$ 

$$\Sigma F_{y} = 0$$
  $\Sigma F_{x} = 0$   $R_{Ay} + R_{By} = 10 \times 3$   $R_{Ax} = 0$   $R_{Ay} = 30 - 13.64$   $R_{Ay} = 16.36 \text{ kN}$ 



# Example 13 (cont.)





$$\frac{x}{16.36} = \frac{3-x}{13.64}$$

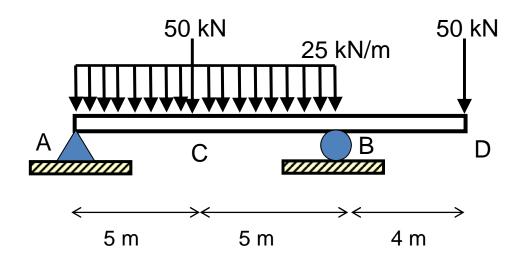
$$13.64x = 49.08 - 16.36x$$

$$30x = 49.08$$

$$x = 1.636$$

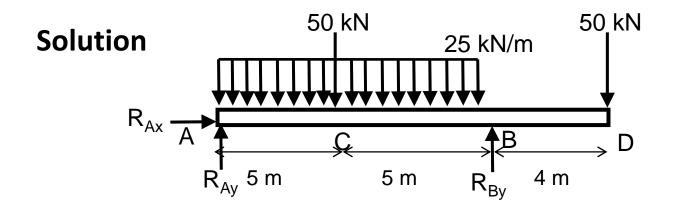








## Example 14 (cont.)



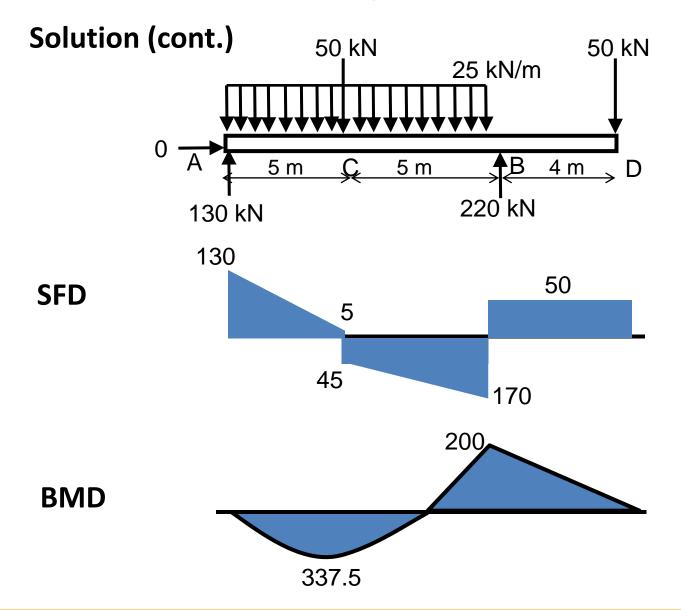
$$\Sigma M_{\rm A} = 0$$
  $\Sigma F_{\rm x} = 0$   $25 \times 10 \times 5 + 50 \times 5 + 50 \times 14 - R_{\rm By} \times 10 = 0$   $R_{\rm Ax} = 0$   $R_{\rm By} = 220 \; \rm kN$ 

$$\Sigma F_y = 0$$
  
 $R_{Ay} + R_{By} = 25 \times 10 + 50 + 50$   
 $R_{Ay} = 350 - 220 = 130 \text{ kN}$ 





# Example 14 (cont.)





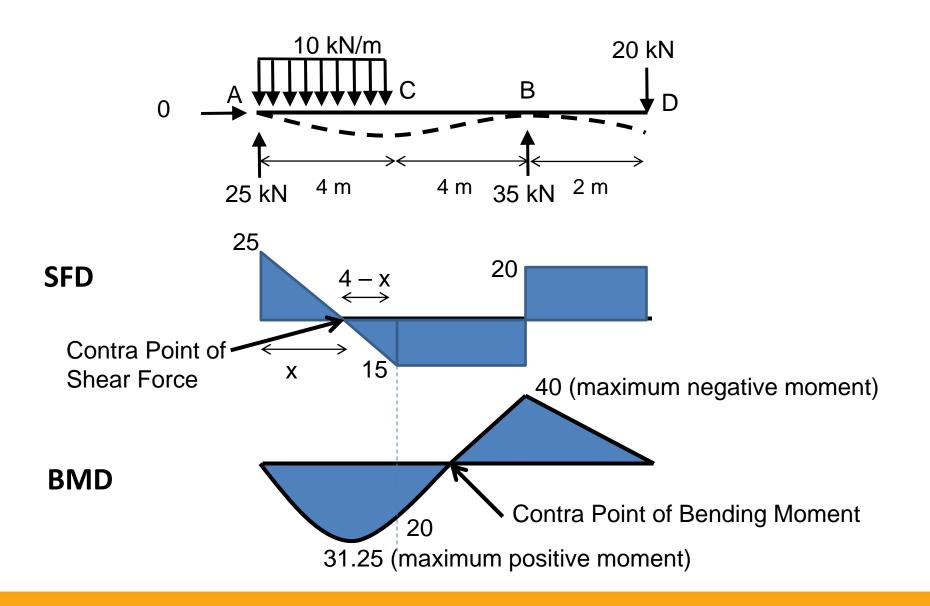
### **Contra Point of SF and BM**

- Contra point is a place where positive shear force/bending moment shifting to the negative region or vice-versa.
- Contra point for shear: **V** = **0**
- Contra point for moment: M = 0
- When shear force is **zero**, the moment is **maximum**.
- Maximum shear force usually occur at the support / concentrated load.





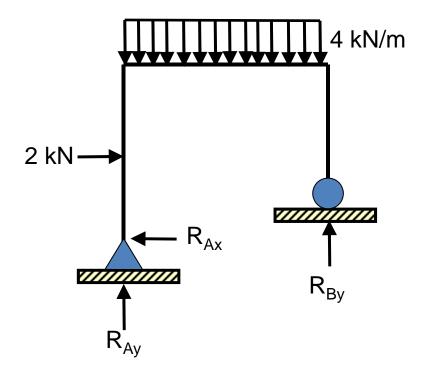
### **Contra Point of SF and BM**





### **Statically Determinate Frames**

 For a frame to be statically determinate, the number of unknown (reactions) must be able to solved using the equations of equilibrium.

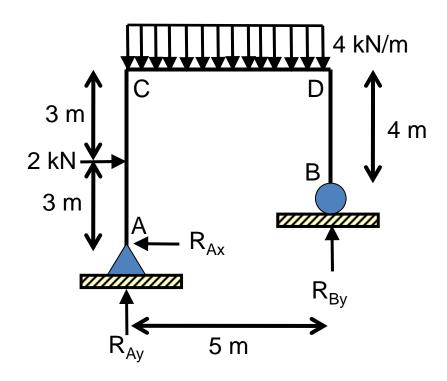


$$\Sigma M_{A} = 0$$
$$\Sigma F_{y} = 0$$
$$\Sigma F_{x} = 0$$





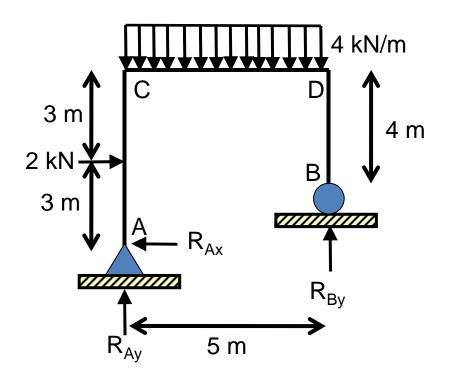
## Example 15





# Example 15 (cont.)

#### **Solution**



$$\Sigma M_{\rm A} = 0$$
  
 $4 \times 5 \times 2.5 + 2 \times 3 - R_{\rm By} \times 5 = 0$   
 $R_{\rm By} = 11.2 \text{ kN}$ 

$$\Sigma F_{y} = 0$$
 $R_{Ay} + R_{By} = 4 \times 5$ 
 $R_{Ay} = 20 - 11.2 = 8.8 \text{ kN}$ 

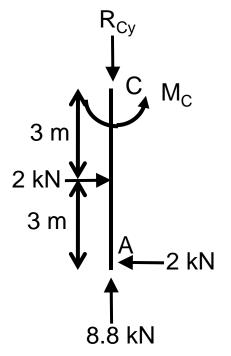
$$\Sigma F_{x} = 0$$

$$R_{Ax} = 2 \text{ kN}$$



# Example 15 (cont.)

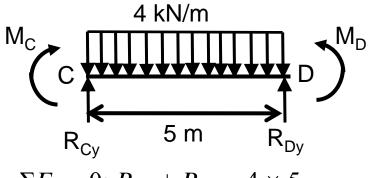
#### Solution (cont.)



$$\Sigma M_{\rm A} = 0: 2 \times 3 - \mathrm{M_C} = 0$$

$$M_C = 6 \text{ kNm}$$

$$\Sigma F_{\rm y} = 0$$
:  $R_{\rm cy} = 8.8 \text{ kN}$ 



$$\Sigma F_y = 0$$
:  $R_{Cy} + R_{Dy} = 4 \times 5$   
 $R_{Dy} = 20 - 8.8 = 11.2 \text{ kN}$ 

$$R_{Dy} = 20 - 8.8 = 11.2 \text{ kN}$$

$$\Sigma M_{\rm C} = 0$$
:

$$M_C + 4 \times 5 \times 2.5 - R_{Dy} \times 5 - M_D = 0$$

$$M_D = 0 \text{ kNm}$$

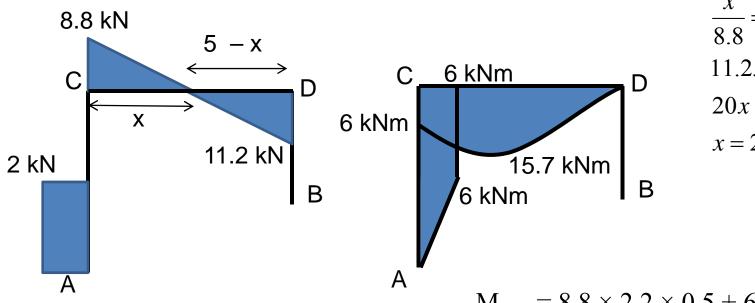
$$\Sigma F_{\rm y} = 0$$
:

$$R_{\rm Dy} = 11.2 \text{ kN}$$



# Example 15 (cont.)

#### **Solution**



$$\frac{x}{8.8} = \frac{5 - x}{11.2}$$

$$11.2x = 44 - 8.8x$$

$$20x = 44$$

$$x = 2.2$$

$$M_{max} = 8.8 \times 2.2 \times 0.5 + 6 = 15.7 \text{ kNm}$$





# References

- 1. Hibbeler, R.C., Mechanics Of Materials, 8th Edition in SI units, Prentice Hall, 2011.
- Gere dan Timoshenko, Mechanics of Materials, 3rd Edition, Chapman & Hall.
- Yusof Ahmad, 'Mekanik Bahan dan Struktur' Penerbit UTM 2001